

February 2, 2026

Hingham Planning Board  
Town of Hingham  
210 Central Street  
Hingham, MA 02043

SLR Project No.: 141.051021.00001

Client Reference No.: 21840.00008

**RE: Comment Response Letter  
Hingham Center for Active Living, Bare Cove Park Drive, Site Plan Review  
Hingham, Massachusetts**

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Dear Planning Board Members:

SLR International Corporation (SLR) is in receipt of correspondence dated January 9, 2026, with comments from Patrick G. Brennan, PE of PGB Engineering, LLC regarding the review of the above-referenced project. We offer the following responses to the comments contained herein.

- C1. The existing and proposed watershed maps are missing from Appendix A of the Stormwater Management Plan. These should be provided so that we can confirm the modeled drainage patterns.
- R1. An updated Stormwater Management Plan has been provided, including the watershed maps.**
- C2. As noted in the Stormwater Management Plan, test holes are required at each of the proposed infiltration facilities to confirm soil textural analysis and depth to seasonal high groundwater. We note that the borings in proximity of the proposed rain garden and infiltration basin IB-2 indicate bedrock at depths close to the bottom elevations of these facilities. Test holes would verify the feasibility of these facilities.
- R2. Test pits were conducted on January 23, 2026, and witnessed by SLR. No groundwater was witnessed and depths to bedrock have been confirmed. See attached Test Pit Summary Plan, which is also included in the updated Stormwater Management Plan. Falling head permeability tests have been performed on soil samples taken during the test pits, and the rates have been applied to the HydroCAD models. The stormwater basins and underground system have been revised to accommodate appropriate separation distances from confirmed bedrock.**
- C3. The drawdown calculations utilize different infiltration rates than those used in the HydroCAD model for the infiltration facilities. These should be consistent and test holes would confirm the appropriate infiltration rates to be used.

- R3. Soil samples were taken during test pits on January 23, 2026. Falling head permeability tests were performed, with results included in the revised Stormwater Management Plan. These new infiltration rates are used in the updated HydroCAD models.**
- C4. We recommend that the open-air infiltration basins have four inches of topsoil (loam) to enhance infiltration. We also recommend that the sediment forebays have 8- to 12- inches of topsoil to limit infiltration.
- R4. Details have been revised on Sheet SD-5 to reflect this recommendation.**
- C5. Additional coordination is required between the Utilities Plan (Sheet UT) and the details. Sheet UT specifies the bottom elevation of the proposed gravel infiltration trenches and refers the reader to Sheet SD-5 for additional information related to the trenches (i.e. the widths). The Gravel Infiltration Trench Detail on Sheet SD-5 shows a varying width of the trenches and refers the reader to Sheet UT for the individual trench widths. A table of trench widths and depths on Sheet SD-5 would be helpful.
- R5. A table of trench widths and depths has been added to Sheet SD-5.**
- C6. We recommend that the filter fabric around the gravel infiltration trenches (including the trenches in the bottoms of the infiltration basins and rain garden) wrap over the top of the 3/4-inch crushed stone layer to keep sediment out of the crushed stone. This will make maintenance easier as only the top twelve inches of 'clean broken stone' will need to be removed and cleaned/replaced when filled with sediment.
- R6. Details have been revised on Sheet SD-5 to reflect this recommendation.**
- C7. We assume that the Riprap Spillway detail on Sheet SD-5 is the spillway for infiltration basin IB-1, however, it is not labeled. This detail, as well as the Forebay Overflow Weir Section Detail (also on Sheet SD-5), should show a concrete cut-off wall (curb or similar) with the top elevation at the design weir elevation so that water does not flow through the stone of the spillways before reaching the design elevation.
- R7. Labels have been added to the details. Additionally, a precast concrete curb has been added to the weir elevations. See Sheet SD-5.**
- C8. The Nyloplast Drain Basin With Dome Grate detail on Sheet SD-5 should specify a sump in the basins (yard drains). We recommend a minimum two-foot sump but deeper sumps would perform better.
- R8. A sump depth has been added to the Nyloplast Drain Basin detail.**
- C9. The drain manhole at the end of the isolator row of the subsurface infiltration system should have outlet inverts specified, with the invert into the isolator row lower than the invert to the manifold leading the other rows of chambers.
- R9. Inverts have been added to the drain manhole on Sheet UT.**



- C10. The proposed inspection ports on the subsurface infiltration system should be a minimum of six-inch diameter to allow for easier inspection and maintenance. Locations of the inspection ports should be shown in plan and there should be one on each end chamber in each row to allow for cleaning.
- R10. The inspection port detail has been revised to reflect a six-inch diameter port. Locations of the inspection ports have been added to Sheet UT.**
- C11. Sheet SD-6 includes details for reinforced concrete (RC) and high-density polyethylene (HDPE) flared end sections. For durability we recommend that all flared end sections be reinforced concrete.
- R11. HDPE flared end section detail has been removed from the Sheet SD-6.**
- C12. The Storm Water Maintenance Program on Sheet UT should be consistent with the Schedule for Inspection and Maintenance (Schedule) included in the Postconstruction Stormwater Management Operation and Maintenance Plan (gravel trenches and yard drains should be included on Sheet UT). The inspection frequencies specified in the Schedule should be utilized with the following recommended changes:
- a. Yard Drains should be inspected and cleaned four times per year, consistent with catch basins.
  - b. Basins and forebays should be mowed regularly with other grass areas to prevent growth of woody vegetation.
- R12. The Stormwater Maintenance Program on Sheet UT and the Postconstruction Stormwater Management Operation and Maintenance Plan have been revised to align. The suggested inspection frequencies have also been added.**
- C13. The Construction Entrance (CE) detail on Sheet SE-2 should specify the minimum length of the construction entrance to be fifty feet.
- R13. A minimum length of 50 feet has been added to the detail on Sheet SE-2.**
- C14. The Erosion Checks described in the Sediment & Erosion Control Specifications on Sheet SE-2 are straw bales, yet the erosion checks specified in plan on Sheet SE-1 and detailed on Sheet SE-2 are compost filter tubes.
- R14. Compost filter tubes will be used. The Sediment & Erosion Control Specifications on Sheet SE-2 have been revised.**



Please feel free to contact me with any additional questions at 203-271-1773.

Regards,

**SLR International Corporation**



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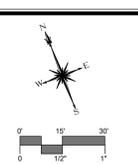
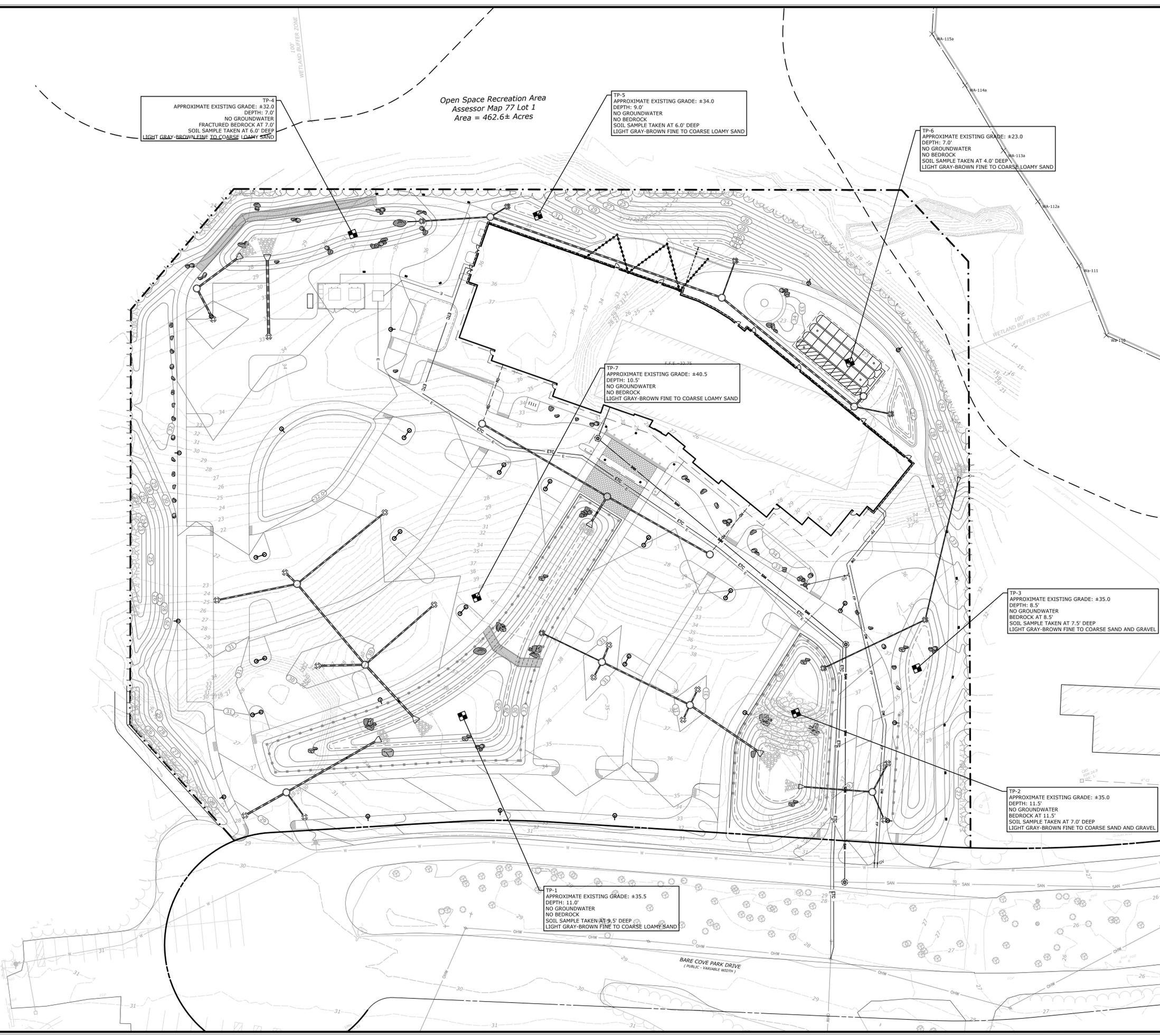
Attachment

141.21840.00008.f226.ltr



**NOTES:**

TEST PITS WERE CONDUCTED ON 1/23/2026 AND WITNESSED BY SLR.



DESCRIPTION	DATE	BY

**TEST PIT SUMMARY PLAN**  
HINGHAM CENTER FOR ACTIVE LIVING  
BARE COVE PARK DRIVE  
HINGHAM, MA

AWG	AWG	TD
DESIGNED	DRAWN	CHECKED
SCALE: 1"=30'		
DATE: JANUARY 30, 2026		
PROJECT NO: 21840.00004		
SHEET NO: 1 OF 1		
SHEET NAME: TP		